

# Soil - Structure Interaction of Multistorey R.C.C. Frames

ALESH MATALE , PROF. AMEY KHEDIKAR

Nagpur, India

## Abstract

*Comprehensive experimental and analytical studies have been carried out to understand the behavior of existing frame buildings constructed before the introduction of seismic design codes in 1970's. Different aspects of the response have been investigated and inherent weaknesses have been pointed out. This usually has been done assuming a fixed-base structure while ignoring the flexibility of soil and foundation. In this thesis, the interaction between the super-structure and sub-structure (SSI) is investigated by modelling the soil as simple as possible to capture the overall response of the system. As new analytical hysteresis rules and more advanced tools of analysis have been developed in recent years, the linear response of a structure which can be representative of a broad range of existing or newly designed structures, is investigated while allowing for flexibility of the soil-foundation system and SSI effects. The use of flexible base in the analysis can lead to reduction in the structural response and damage consequences in joints and infills.*

*The results of this study suggest that the compliance of simply modelled soil for typical building structures have in average beneficial effects in terms of structural demand especially for stiff structures.*

**Keywords** :- Soil structure interaction, Linear analysis, Shear wall, Soft soil.

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## Introduction

A common design practice for dynamic loading assumes the building to be fixed at their bases. In reality the supporting soil medium allows movement to some extent due to its property to deform. This may decrease the overall stiffness of the structural system and hence may increase the natural periods of the system, such influence of partial fixity of structures at foundation level due to soil flexibility intern alters the response. On the other hand, the extent of fixity offered by soil at the base of the structure depends on the load transferred from the structure to the soil as the same decides the type and size of foundation to be provided. Such an interdependent behavior of soil and structure regulating the overall response is referred to as soil structure interaction.

This effect of soil flexibility is to be accounted through consideration of springs of specified stiffness. Thus the change in natural period due to effect of soil structure interaction may be an important issue from the viewpoint of design considerations. Also it is usual practice to treat the brick infill as a non-structural element and therefore all the lateral loads are assumed to be resisted by the frame, but performance of buildings in the recent earthquakes (e.g: 1985 Mexico City earthquake, 2001 Bhuj earthquake) clearly illustrates that the presence of infill wall has significant structural implication. Therefore, the structural contribution of infill wall cannot simply be neglected particularly in regions of moderate and high seismicity where the frame infill interaction may cause substantial increase in both stiffness and strength. A review of analysis and design provisions related to masonry infill RC frames in seismic design codes of different countries show that only a few codes have considered the effect of infill in analysis and design of masonry in filled RC frames. On the other hand, the stiffness and strength of in filled frames with openings are not taken care of by most of the codes. Hence the behavior of in filled frames with opening needs to be studied extensively in order to develop a rational approach or guidelines for design.

In the last three decades, the effect of SSI on earthquake response of structures has attracted an intensive interest among researchers and engineers. Most of these researches focus on theoretical analysis, while less has been done on the experimental study. The interaction among the structure, foundation and soil medium below the foundation alter the actual behaviour of the structure considerably as obtained by the consideration of the structure alone. Flexibility of soil medium below foundation decreases the overall stiffness of the building frames resulting in an increase in the natural period of the system.

## Review of Literature

**Badry, P. and Satyam, N. (2016)** Seismic damage surveys and analyses conducted on modes of failure of structures during past earthquakes observed that the asymmetrical buildings show the most vulnerable effect throughout the course of failures.

**Lu Y., et al. (2016)** a comprehensive parametric study has been carried out to investigate the seismic performance of multistory shear buildings considering soil–structure interaction (SSI). More than 40,000 SDOF and MDOF models are designed based on different lateral seismic load patterns and target ductility demands to represent a wide range of building structures constructed on shallow foundations.

**Mitropoulou C. C., et al. (2016)** 3D reinforced concrete (RC) and steel building structures have been considered for studying the effect of soil–structure interaction modelling on the structural fragility assessment of such structures.

**Shanmugam et al. (2015)** the effect of soil–structure interaction on a four storeyed, two bay frame resting on pile and embedded in the cohesive soil was examined.

**Li et al. (2014)** Taking the Shanghai Tower with a total height of 632 m as the research object, the substructure approach is used to simulate the SSI effect on the seismic responses of Shanghai Tower.

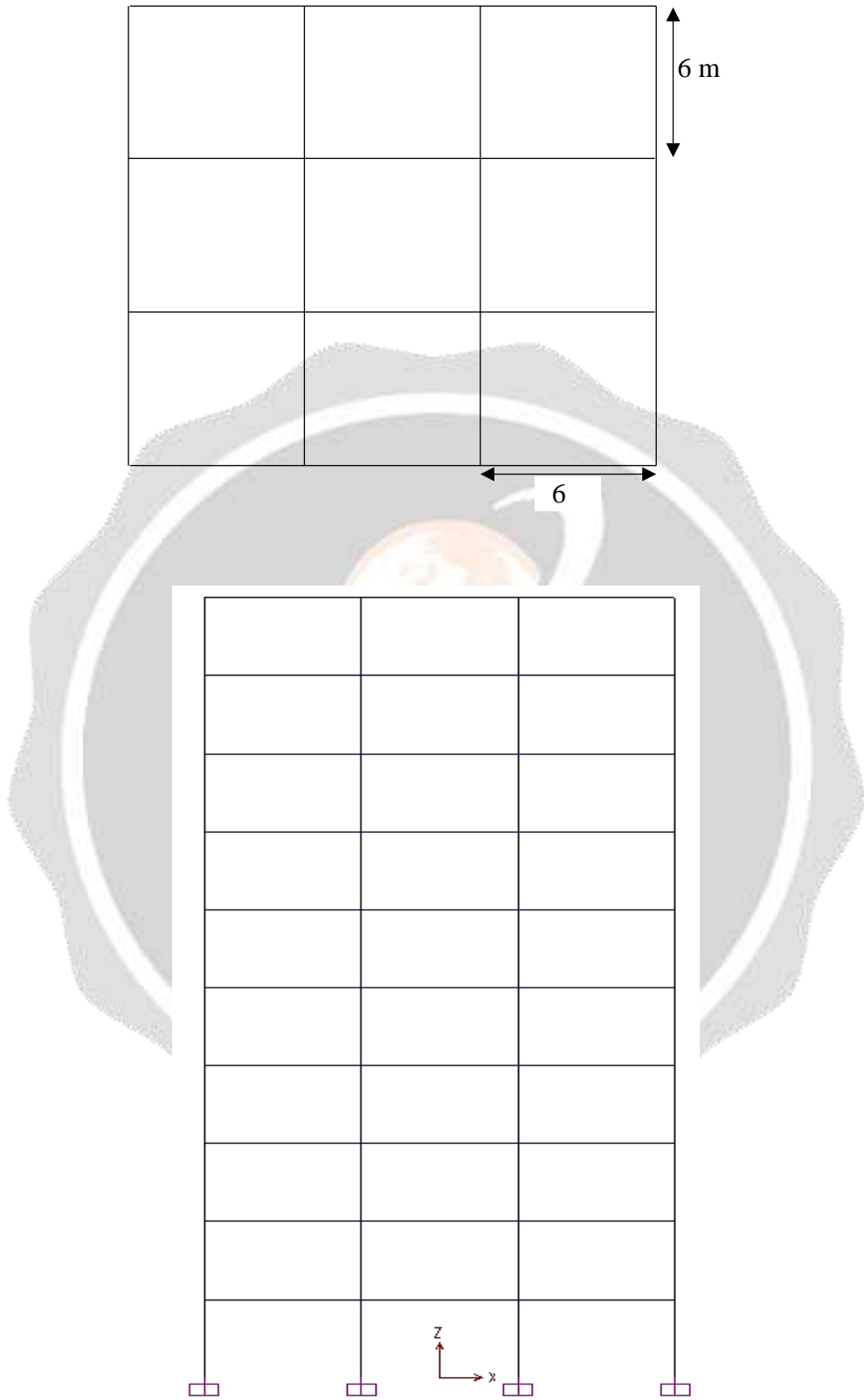
## Mathematical Modelling

### Idealization of Structure

To study the dynamic behavior of building structure while considering the effect of soil structure interaction, building frame is modeled as 3D space frame using standard two noded frame element with two longitudinal degrees of freedom and one rotational degree of freedom at each node. At the interface of infill and frame, the infill element and the frame element are given same nodes.

The idealized form of a typical 3 bay x 3 bay 10 storey building frame with infill wall modeled as represented schematically in figure the present study also considers bare frame to see how correctly the influence of soil structure interaction on dynamic behavior can be predicted. This may give an idea about the error, which one should liable to commit if this popular but grossly inaccurate approach is invoked.

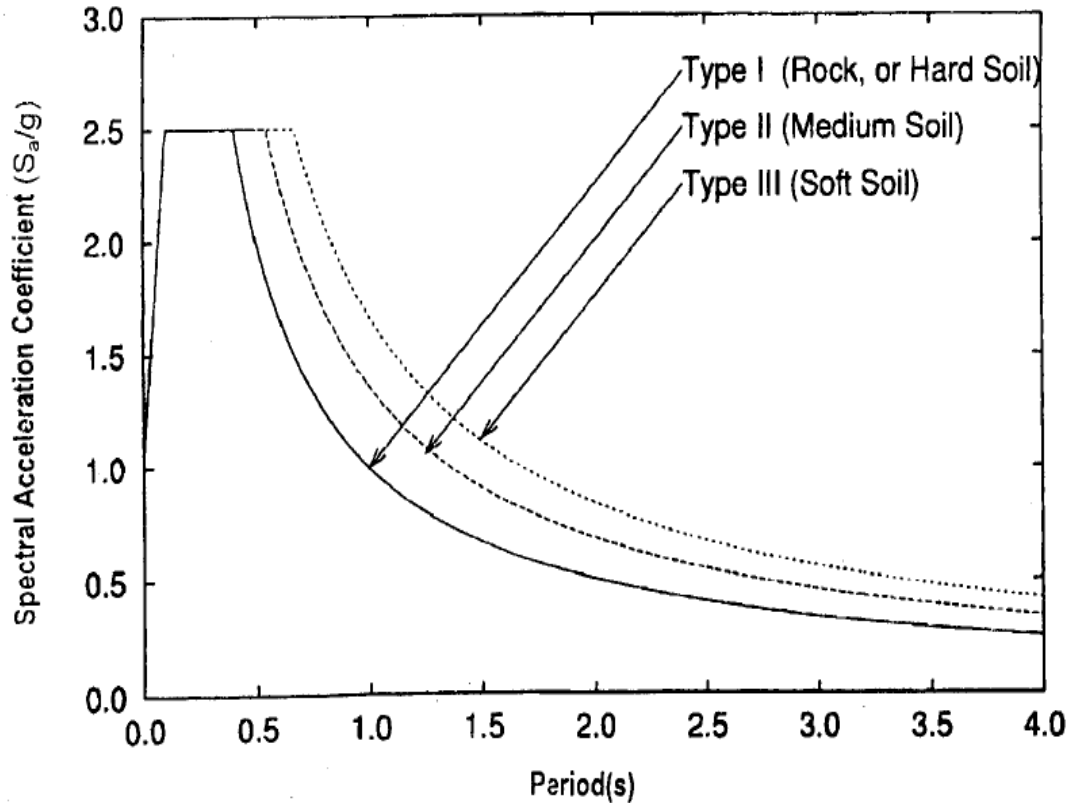
A 3 bay x 3 bay building frames with 10 storey's on isolated footing have been considered. The height of each storey is taken as 3.6 m and the longitudinal and transverse dimensions of 3 bays x 3 bay building is taken as 6 m for central bay and 6 m for the two side bays. For all the buildings the dimensions of reinforced concrete column are taken as 600 x 600 mm and for beam it is 200 x 600 mm. Similarly thickness for roof and floor is taken as 150 mm and their corresponding dead load is directly applied on the beam. The brick infill with thickness 150 mm. All the above dimensions were arrived on the basis of the design following the respective Indian code for design of reinforced concrete structure. However, these design data are believed to be practicable and hence, do not affect the generality of the conclusion.



**Plan and Elevation of building**

## Idealization of Soil

Flexibility of soil medium below foundation may appreciably alter the natural periods of any building. It usually causes to elongate time period of structure. It is observed (from the fig ) that soft soil amplifies ground motion more than that of hard soil.

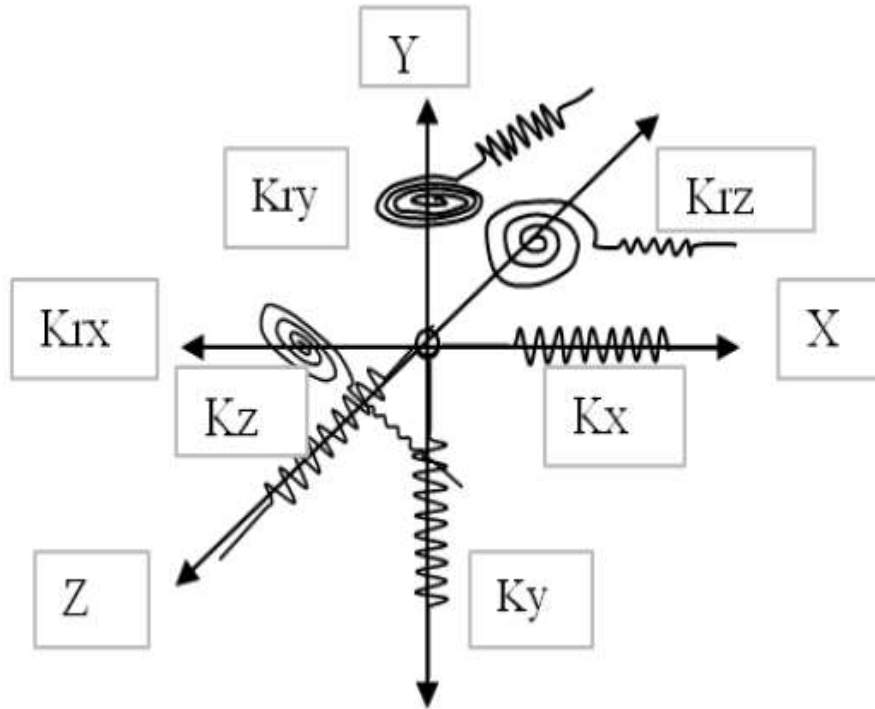


Response spectra for rock and soil sites for 5% damping (IS1893)

## Soil stiffness for six direction springs

The flexibility of soil is usually modelled by inserting springs between the foundation member and soil medium. While modelling, the number of degree of freedom should be selected carefully considering the objective of the analysis. During earthquake a rigid base may be subjected to a displacement in six degrees of freedom, and therefore resistance of soil can be expressed by the six corresponding resultant force components. Hence to make the analysis most general, translations of foundation in two mutually perpendicular principle horizontal directions and vertical direction as well as rotation of the same about these three directions are considered in this study.

In this project is where the effect of soil flexibility is not considered for isolated footing it is named as case (a). To simulate the effect of soil flexibility, three translation springs along two horizontal and one vertical axis, together with three rotational springs about those mutually perpendicular axes, have been attached as shown in Fig is case (b). The expressions for these spring stiffness are presented in table



Idealization arrangement at a typical column square foundation strip and equivalent soil spring junction [Case (b)]  
Spring stiffness for square footing along various degrees of freedom

Degrees of freedom	Stiffness of equivalent soil spring
Vertical $K_y$	$4.54Gb/(1-\mu)$
Horizontal (lateral direction) $K_x$	$9Gb/(2-\mu)$
Horizontal (longitudinal direction) $K_z$	$9Gb/(2-\mu)$
Rocking (about the longitudinal) $K_{rx}$	$0.45Gb^3/(1-\mu)$
Rocking (about the lateral) $K_{rz}$	$0.45Gb^3/(1-\mu)$
Torsion $K_{ry}$	$8.3Gb^3$

Note: “b” is half width of a square foundation

To obtain the values of spring stiffness’s of the springs for hard, medium and soft soil, value of shear modulus (G) of soil have been estimated using the following empirical relationship.

$$G=120 N^{0.8} \text{ t/ft}^2 \dots\dots\dots 1$$

$$G=13333.33 N^{0.8} \text{ KN/m}^2 \dots\dots\dots 2$$

Where, N= Number of blows to be applied in Standard Penetration Test (SPT) of the soil; and the poisons ratio ( $\mu$ ) of the soil has been taken to be equal to 0.3 for all types of clay. N is taken as 3, 6, and 30 for soft, medium and hard soil respectively. The details of different soil parameters are tabulated in table Safe Bearing Pressure is assumed for footing placed at depth 1.4 m below ground level. Since the stiffness’s of spring used to represent the soil flexibility are highly sensitive to the size of footing below which they are attached to, dimensions of various footing have been very rigorously computed separately based on Safe bearing capacity to have an exhaustive idea. The load carried by each column is obtained from the response spectrum analysis using SAP2000. All isolated footings are

assumed to be square in shape. Table gives the values for spring stiffness determined using the above formulas for a ten storey building designed with isolated square footing.

### Types of soils and their parameters

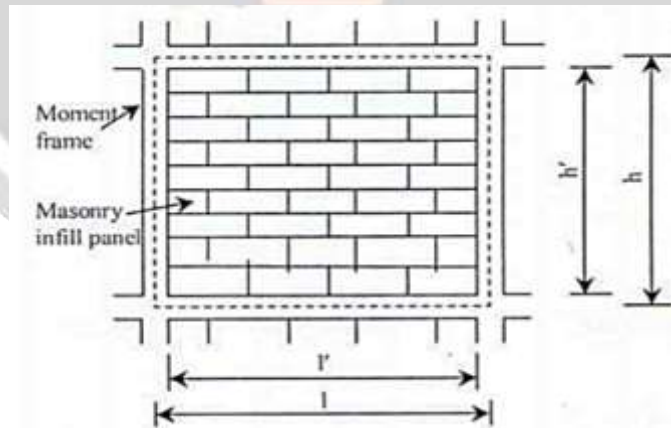
Sr.No.	Type of soil	N value	Shear Modulus in KN/m <sup>2</sup>
1.	Soft	3	32109.66
2.	Medium	6	55906.1
3.	Hard	30	202598.2

### Infill without opening

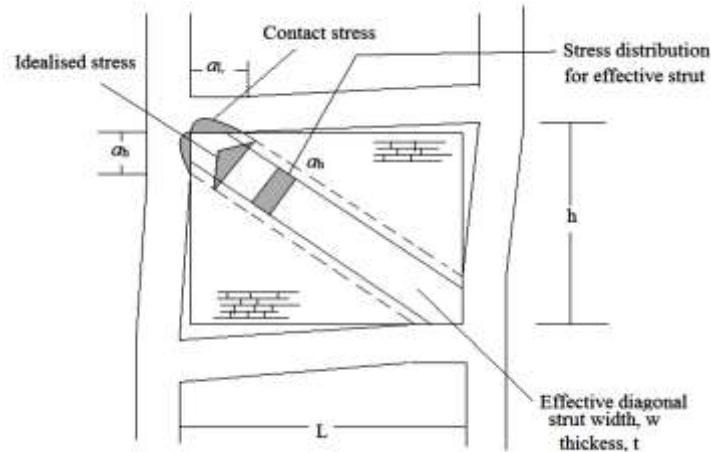
Infill masonry is the building of structure from which individual unit laid and bond together by mortar; the term masonry can also refer to the unit themselves. The common material of masonry construction are brick, stone, Marble, Granite, Limestone, Cast iron, Concrete block, Glass block and tile masonry is generally highly durable form of construction .

Existence infilling is noted to increase the ultimate lateral resistance of the system while resulting in less ultimate lateral deflection for lower infilling. The study of interaction of infill with the frame has been attempted by using sophisticated analysis like finite element analysis or theory of elasticity. But due to uncertainty in defining the inter face condition between the in filled with frame, an approximate analysis method may be better acceptable. One of the most common approximations of in filled wall is on the basis of equivalent diagonal strut, i.e. the system is modeled as a brace frame and infill walls as web element for the strut member. We required width and thickness of equivalent diagonal strut that will found out by using

- 1) Infill wall without opening
- 2) Infill wall with opening



**Figure: Masonry infill panel in frame structure Infill wall without opening**



### Equivalent diagonal strut

Width and thickness of strut depend upon length of contact between wall and column ( $\alpha h$ , and  $\alpha L$ .) The value  $\alpha h$ , and  $\alpha L$  on the basis of beam elastic boundary following equation proposed to determine  $\alpha h$ , and  $\alpha L$ .

$$\alpha_h = \frac{\pi^4}{2} \sqrt{\frac{E_f I_c h}{2E_m t \sin 2\theta}}$$

$$\alpha_L = \pi^4 \sqrt{\frac{E_f I_b h}{E_m t \sin 2\theta}}$$

Where,

$h$  = height of masonry infill panel, m.

$L$  = length of infill panel, m.

$t$  = thickness of infill panel and equivalent strut, m.

$E_f$  = modulus of elasticity of frame material, MPa

$E_m$  = modulus of elasticity of infill material, MPa

$I_c$  = moment of inertia of column,  $m^4$ .

$I_b$  = moment of inertia of beam,  $m^4$ .

$\theta$  = angle whose tangent is the infill height-to-length aspect ratio, radians.

The following equation to determine the equivalent or effective strut width  $w$ , where the strut is assumed to be subjected to uniform compressive stress.

$$W = \frac{1}{2} \sqrt{\alpha_h^2 + \alpha_L^2}$$

Frame properties

Width of Beam and column=0.23m

Depth of Beam and column=0.40m

By using value of column and beam, we calculate width of infill

- 1) Infill-applying infill in terms of strut having width=0.7m and Thickness= 0.23 m
- 2) With having following property and with pinned joint Elastic modulus=700 Mpa
- 3) Unit weight=18.85kN/m<sup>3</sup>
- 4) Poisons ratio= 0.3

## Result and Analysis

### Linear Static Analysis

The plan layout and elevation of G+10 storey building is shown in fig. The building is deliberately kept symmetric in plan along both orthogonal directions. The building considered is assumed to be located in Seismic Zone II.

#### Data

- 1) Live Load = 3.5 kN/m<sup>2</sup> at typical floor, 1.5 kN/m<sup>2</sup> at terrace
- 2) Floor finish = 1 kN/m<sup>2</sup>
- 3) Terrace finish = 1 kN/m<sup>2</sup>
- 4) Location = Nagpur city
- 5) Earthquake load = As per IS-1893(Part-1)-2002
- 6) Storey height = 3.6 m
- 7) Walls = 0.15 m thick
- 8) Column size = 0.6 X 0.6 m
- 9) Beams = 0.2 X 0.6 m
- 10) Slab thickness = 0.15 m
- 11) Density of concrete = 25 kN/m<sup>3</sup>
- 12) Density of brick = 20 kN/m<sup>3</sup>
- 13) Seismic zone = II

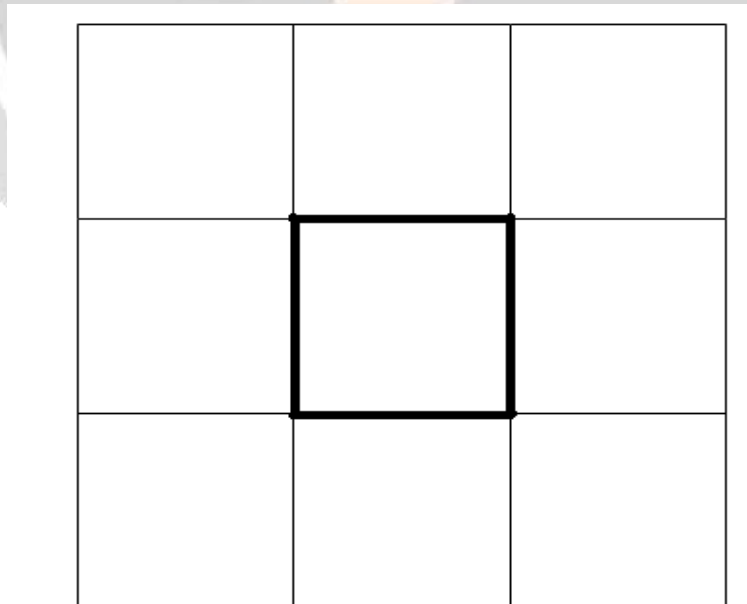
#### Bare frame (Soft soil)




**Analysis results of Structure type A for soft soil**

Model.	Storey	Max Reaction in kN	Max.Displacement of top storey In mm	Frequency In Cyc/sec.	Period In Sec.	Base shear in kN
SAa (Without SSI)	G+10	5239	69	0.46	2.1	353
				0.46	2.1	
				0.53	1.8	
SAb (With SSI)	G+10	4932	73	0.518	2.1	343
				0.518	2.1	
				0.608	1.8	
SAb' (With SSI)	G+10	4319	97	0.443	2.5	305
				0.443	2.5	
				0.640	1.9	

**Frame with Shear wall considering its stiffness (Soft soil)**



**Table 5.4 Analysis results of Structure type B, case 2' for soft soil**

Case No.	Storey	Max Reaction in KN	Max.Displacement of top storey In mm	Frequency In Cyc/sec.	Period In Sec.	Base shear in KN
SB2'a (Without SSI)	G+10	5758	40	0.75	1.3	436
				0.84	1.1	
				0.84	1.1	
SB2'b (With SSI)	G+10	5248	50	0.60	1.6	412
				0.64	1.5	
				0.62	1.59	
SB2'b' (With SSI)	G+10	4558	74	0.4	2.4	291
				0.4	2.4	
				0.53	1.8	

## Conclusions

In the present study, the effect of soil structure interaction on the dynamic characteristics of structure has been studied. Some of the conclusions that can be drawn from the observations made above are,

1. The study shows that consideration of different parameter such as soil structure interaction, and location of walls influences time period, displacement and base shear of building frame considerably. Hence it is important to consider to all these parameters in the analysis of structures.
2. Shear walls located in the central part of the multistoried building gives lesser displacement and more base shear compared to other locations.

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**Authors and Biography**

